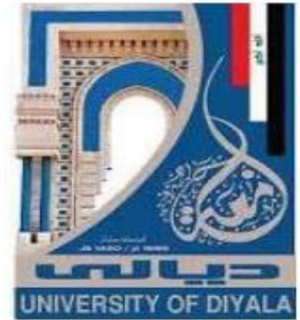


**Ministry of Higher Education
and Scientific Research
University of Diyala
College of Engineering**



Behavior of Single Pile Subjected to Dynamic Loading in Gypseous Soil

**A Thesis Submitted to the Council of the College of
Engineering / University of Diyala in Partial Fulfillment of the
Requirements of the Degree of Master of Science in Civil
Engineering**

**By
Noor Deia Abd
B.SC. Civil Engineering, 2016**

**Supervisor by
Assist. Prof. Dr. Safa Hussain Abid Awn**

August 2020 A.D.

IRAQ

Dhul-Hijjah1441 A.H.

بِسْمِ اللَّهِ الرَّحْمَنِ الرَّحِيمِ

﴿ وَقُلْ اَعْمَلُوا فَسَيَرَى اللَّهُ عَمَلَكُمْ
وَرَسُولُهُ وَالْمُؤْمِنُونَ وَسَتُرَدُّونَ اِلَى
عَالِمِ الْغَيْبِ وَالشَّهَادَةِ فَيُنَبِّئُكُمْ بِمَا كُنْتُمْ
تَعْمَلُونَ ﴾

صدق الله العلي العظيم (التوبة 105)

Dedication

***To my ideal life..... my father
To the light of my eyes..... my mother
To the candles that light my path... my
brother and my sisters
To everyone who supported me to
complete my thesis in the best form***

Noor

Acknowledgments

In the name of God, the Most Gracious, the Most Merciful, and prayers and peace be upon our Prophet Muhammad. I thank God who gave me the strength to complete my work.

I thank my supervisor, the assistant professor. Dr. Safa Hussein, for his valuable advice.

I also extend my sincere thanks to all my professors in the Department of Civil Engineering in the undergraduate and master's levels.

As I especially thank my colleagues who supported me throughout my studies.

Finally, my sincere thanks and gratitude to my family, who have the greatest role of support.

Noor D. Abd Al-tamimi

COMMITTEE DECISION

We certify that we have read the thesis entitled (**Behavior of single pile subjected to dynamic loading in gypseous soil**) and we have examined the student (**Noor Deia Abd**) in its content and what is related with it, and in our opinion it is adequate as a thesis for the Degree of Master of Science in Civil Engineering.

Examination Committee

Signature

Assist. Prof. Dr. Safa Hussain Abid Awn (Supervisor)

Prof. Dr. Mohammed Yousif Fattah (Chairman)

Assist. Prof. Dr. Hassan Obaid Abbas (Member)

Lect. Dr. Qutaiba Gazi Majed (Member)

Prof. Dr. Khattab Saleem Abdul-Razzaq.....(Head Department)

The thesis was ratified at the Council of College of Engineering/ University of Diyala.

Signature:.....

Name: Prof. Dr. Anees Abdullah khadom

Dean of College of Engineering/ University of Diyala.

Date:

CERTIFICATION

I certify that the thesis entitled “**Behavior of single pile subjected to dynamic loading in gypseous soil**” is prepared by “**Noor Deia Abd**” under my supervision at the Department of Civil Engineering- College of Engineering- Diyala University in a partial fulfillment of the Requirements for the Degree of **Master of Science in Civil Engineering**.

Signature:

Supervisor: Assist. Prof. Dr. Safa Hussain Abid Awn

Date: / / 2020

In view of the available recommendation, I forward this thesis for debate by the examining committee.

Signature:

Name: Prof. Dr. Khattab Saleem Abdul-Razzaq

Chairman of the Department of Civil Engineering.

Date: / / 2020

SCIENTIFIC AMENDMENT

I certify that this thesis entitled “**Behavior of single pile subjected to dynamic loading in gypseous soil**” presented by “**Noor Deia Abd**” has been evaluated scientifically, therefore, it is suitable for debate by examining committee.

Signature.....

Name: Assist. Prof. Dr. Mohammed Faiq Aswad

Address: University of Al-Kufa / College of Engineering

Date:

LINGUISTIC AMENDMENT

I certify that this thesis entitled “**Behavior of single pile subjected to dynamic loading in gypseous soil**” presented by “**Noor Deia Abd**” has been corrected linguistically, therefore, it is suitable for debate by examining committee.

Signature.....

Name: Assist. Dr. Amjad Latif Jabbar

Address: University of Diyala/ College of Education for Human Science

Date:

Abstract

Behavior of Single Pile Subjected to Dynamic Loading in Gypseous Soil

By

Noor Deia Abd

Supervisor

Assist. Prof. Dr. Safa Hussain Abid Awn

In this thesis, the dynamic response of single pile with a static load is studied under the dynamic load that were generated by use of an electric rotary motor with eccentric loading installed above cap pile, the tests are performed under dry and soaked states.

Many parameters are taken into consideration, including slenderness ratio (L/D) 12, 17, 22, 27 for solid steel pile with frequency 10 Hz and gypsum content 30%, different frequencies 10, 15, 20, 25 for solid steel pile with (L/D) 27, gypsum content 65% for solid steel pile, also (L/D) 27 and frequency 10 Hz, types of pile (concrete pile, hollow steel pile, rough steel pile, timber pile) with (L/D) 27 and frequency 10 Hz in all tests.

The results of the tests showed that the velocity of vibration, acceleration of pile, displacement amplitude and settlement decrease with an increase in slenderness ratio and decrease in soaked soil compared to the corresponding values in dry soil where velocity decreased when increasing the slenderness ratio from 12 to 27 in the dry and soaked states by 78.2% and 77.2% respectively. Acceleration decreased by 84% and 80%, respectively, while the displacement amplitude decreased by 66.67% and 61.4%, respectively. As well, settlement decreased by 31.25%.

The tests also showed that the velocity of vibration, acceleration, displacement amplitude of the solid steel pile with L/D 27 constant in all tests increased with increasing the frequency of the vibration source as the velocity decreased when the frequency is reduced from 25 to 10 Hz in the dry and soaked condition by 98% and 97.8% respectively, acceleration decreased by 98% and 97.8% respectively. While the displacement amplitude decreased by 84.7% and 80% respectively and the settlement is decreased by 31.25%.

In addition, the results of tests showed that the displacement amplitude, velocity of vibration and acceleration increased with an increase in the gypsum content in soil, as the velocity decreased when decreasing the gypsum content from 65% to 30% in the dry and soaked states by 58.85% and 50% respectively. Acceleration decreased by 29.4% and 20%, respectively. While the displacement amplitude decreased by 20% and 30.5%, respectively and settlement decreased by 12%.

The results also showed that the velocity of vibration, acceleration and displacement amplitude are differed according to the type of pile. The dynamic response showed that an increase in timber pile compared to other type due to low hardness, while the settlement is less than in other piles as a result of friction between pile and soil.

TABLE OF CONTENTS

Heading	Page
Acknowledgments	
ABSTRACT.....	I
TABLE OF CONTENTS.....	III
LIST OF TABLES.....	VIII
LIST OF FIGURES.....	X
LIST OF PLATES.....	XV
LIST OF Abbreviations.....	XVI
CHAPTER ONE	1
INTRODUCTION.....	1
1.1 General.....	1
1.2 Problems with Gypseous Soils.....	2
1-3 Dynamic Loading.....	3
1.4 Types of Dynamic Loads.....	4
1.5 Types of Machines	5
1.6 Objective of the Study.....	5
1.7 Layout of the Study.....	6
CHAPTER TWO.....	7
REVIEW OF LITERATURE.....	7
2.1 Introduction.....	7
2.2 Gypseous Soil.....	7
2.2.2 Characteristics of Gypsum.....	9
2.2.2.1Cementation.....	9
2.2.2.2 Solubility and rate of Dissolution.....	9
2.3 Distribution of Gypseous Soil in Iraq.....	10
2.4 Collapsibility of Gypseous soils.....	11
2.4.1 Collapse Mechanism	13
2.5 Effect of Soaking on Engineering Properties of Gypseous soils.....	14

2.5.1 Effect of Soaking on Collapsibility.....	15
2.6 Pile Foundation.....	18
2.6.1 Uses of Piles.....	18
2.6.2 Types of Piles.....	18
2.7 Method of Installation.....	20
2.8 Steel Pile.....	22
2.9 Timber Piles.....	24
2.10 Concrete Pile.....	25
2.11 Pile Capacity.....	27
2.12 Dynamic Load.....	30
2.12.1 Introduction to Dynamic Loading of Soil.....	30
2.12.2 Factors Affecting Soil Behavior Under Dynamic Loading.....	31
2.13 Types of Damping.....	32
2.14 Design Criteria of Machine Foundations	32
2.15 Pile under Vertical Vibration.....	35
2.16 General Comments.....	39
CHAPTER THREE.....	40
EXPERIMENTAL WORK.....	40
3.1 Introduction.....	40
3.2 Gypseous Soil Site Description and Sampling.....	40
3.3 Characterization of Gypseous Soil.....	41
3.3.1 Physical Tests.....	41
3.3.1.1 Grain Size Distribution.....	42
3.3.1.2 Specific Gravity (Gs).....	45
3.3.1.3 Moisture Content, (ω)%.....	45
3.3.1.4 Atterbery Limit.....	45
3.3.1.5 Compaction Test.....	46
3.3.1.6 Relative Density, (D_r %)......	47
3.3.1.7 Identification of Gypsum Content (G.C%).....	48
3.3.2 Chemical Tests.....	49
3.3.3 X-Ray Diffraction (XRD) Analysis.....	50
3.3.4 Engineering Tests.....	50

3.3.4.1 Single – Collapse Test.....	50
3.3.4.2 Direct Shear Test.....	52
3.4 Design Details of Model and Material Used.....	53
3.4.1 Type of piles used in this study.....	53
3.4.2 Steel Model.....	55
3.4.3 Density control.....	55
3.4.4 Source of Vibration and Equipment for Inducting Vibration.....	56
3.4.5 Derives for measuring vibration response	59
3.4.6 Equipment and Devices.....	60
3.5 Jack System.....	61
3.6 Preparation and Test Procedure.....	63
3.7 General Remarks.....	65
CHAPTER FOUR.....	68
RESULTS AND DISCUSSION	68
4.1 Introduction.....	68
4.2 Results of Vibration Test for Different Slenderness Ratio of Steel Solid Pile in Dry and Soaked State with Gypsum Content 30%.....	68
4.2.1 Results from Vibration Test on Dry Soil.....	68
4.2.2 Results from Vibration Test on Soaking Soil.....	73
4.2.3 Settlement from Vibration Test on Dry and Soaking Soil at different slenderness ratios.....	78
4.3 Results of Vibration Test of Single Steel Solid Pile L/D=27 for Different Frequency in Dry and Soaked State with gypsum content 30%	80
4.3.1 Results from Vibration Test on Dry Soil.....	80
4.3.2 Results from Vibration Test on Soaking Soil.....	86
4.3.3 Settlement from Vibration Test on Dry and Soaking Soil at Single Steel Solid Pile with Different Frequency	90
4.4 Results of Vibration Test of Single Solid Steel Pile L/D=27 for Frequency 10 Hz in Dry and Soaked State with Gypsum Content (30,65%).....	91
4.4.1 Results from Vibration Test on Dry Soil.....	91
4.4.2 Results from Vibration Test on Soaking Soil.....	94
4.4.3 Settlement from Vibration Test on Dry and Soaking Soil at Single Solid Steel	

Pile for Frequency 10Hz with Two Gypsum Content.....	96
4.5 Results of Vibration Test of Different Type of Pile that L/D=27 for Frequency 10 Hz In Dry and Soaked States.....	97
4.5.1 Results from Vibration Test on Dry Soil.....	97
4.5.2 Results from Vibration Test on Soaking Soil	100
4.5.3 Settlement from Vibration Test on Dry and Soaking Soil at Five Types of Piles for Frequency 10 Hz	103
4.6 Dynamic Response to the Slenderness Ratio at Dry and Soaked State.	105
4.6.1 The Relation Between the Maximum Velocity and the Slenderness Ratio for Solid Steel Pile).....	105
4.6.2 Relation Between the Maximum Acceleration and the Slenderness Ratio for Solid Steel Pile.....	106
4.6.3The Relation Between the Maximum Displacement Amplitude and the Slenderness Ratio for Solid Steel Pile.....	107
4.6.4 Reduction Factor in Settlement Value at Different Values of Slenderness Ratio (L/D) with Frequency 10 Hz in Dry and Soaked State.....	108
4.7 Dynamic Response to Frequency for steel solid pile that (L/D=27) Dry and Soaked State.....	109
4.7.1 The Relationship Between the Maximum Velocity and Different Frequencies for Solid Steel Pile (L/D=27)	109
4.7.2 The Relationship Between the Maximum Acceleration and Different Frequencies for Solid Steel Pile (L/D=27)	110
4.7.3 The Relationship Between the Maximum Displacement Amplitude and Different Frequencies for Solid Steel Pile (L/D=27).....	111
4.7.4 Reduction Factor in Settlement Value at Different Values of Frequencies for Solid Steel pile (L/D=27) in Dry and Soaked State.....	112
4.8 Comparison Between Two Soils With Two Different Percentages of Gypsum Content (30,65)% at Frequency 10 Hz in Dry and Soaked State for Solid Steel Pile (L/D 27)....	112
4.8.1 Velocity, Acceleration and Displacement Amplitude of Solid Steel Pile with Gypsum Content (30,65) in Dry and Soaking State.....	112
4.8.2 Reduction Factor in Settlement Value for Solid Steel Pile in Dry and Soaked at Frequency 10 Hz with G.C (30,65)%.....	113

4.9 Comparison Between Different Types of Piles at Frequency 10 Hz and L/D 27 with G.C 30% in Dry and Soaked State.....	114
4.9.1 Compare Between Different Types of Piles in Terms of Velocity	114
4.9.2 Compare Between Different Types of Piles in Terms of Acceleration.....	114
4.9.3 Compare Between Different Types of Piles in Terms of Displacement Amplitude	115
4.9.4 Compare Between Different Types of Piles in Terms of Settlement.....	116
4.10 The Results of Static Load at Different Slenderness Ratio for Solid Steel Pile in Dry State	117
CHAPTER FIVE	118
CONCLUSIONS AND RECOMMENDATIONS.....	118
5.1.Conclusions	118
5.2.Recommendations.....	122
• REFERENCES	123
APPENDIX A	A-1

LIST OF TABLES

NO.	Titles	Page No.
(2.1)	Classification of gypseous soil by Barazanji (1973).	8
(2.2)	Classification of gypseous soil by Nashat (1990).	8
(2.3)	The Collapse Potential Severity Problem, (Al-Obaidi, 2014).	13
(2.4)	A some of researcher's conclusions about the soaking effect on the collapsibility.	17
(2.5)	Pile Classification.	19
(2.6)	Permissible amplitudes of vibration according to Barkan (1962).	34
(3.1)	Results of Physical Properties of Two Samples of Soils.	44
(3.2)	Results of Chemical Properties of Two Samples based on (BS 1377: 1990, Part 3).	50
(3.3)	The Collapse Potential of Two Samples Used.	52
(3.4)	Results of Conventional Direct Shear Test of Two Specimens.	53
(3.5)	The properties of piles used.	55
(4.1)	Reduction factor in maximum velocity magnitude at different slenderness ratio (dry and soaked state).	104
(4.2)	Reduction factor in maximum acceleration magnitude at different ,slenderness ratio (dry and soaked state).	107
(4.3)	Reduction factor in maximum displacement amplitude value at different slenderness ratio (dry and soaked state).	108
(4.4)	The Reduction Factor in Settlement Value When Increasing the Slenderness ratio of solid steel pile with frequency 10 Hz.	108
(4.5)	Reduction factor in maximum velocity value at different frequency ratio (dry and soaked state).	109
(4.6)	Reduction factor in maximum acceleration magnitude at different frequency (dry and soaked state).	110
(4.7)	Reduction factor in maximum displacement amplitude value at different frequency ratio (dry and soaked state).	111
(4.8)	Reduction factor of the settlement value when increasing frequency.	112

(4.9)	Reduction factor in maximum velocity, acceleration and displacement amplitude at frequency 10 HZ with G.C (30, 65)% in dry and Soak state.	113
(4.10)	Reduction factor of the settlement value when increasing gypsum content from 30% to 65% in dry and soak state .	113
(4.11)	Reduction factor in maximum velocity in dry and soak state for different types of piles.	114
(4.12)	Reduction factor in maximum acceleration in dry and soak state for different types of piles.	115
(4.13)	Reduction factor in maximum amplitude in dry and soak state for different types of piles.	115
(4.14)	Reduction factor in maximum settlement in dry and soak state for different type of piles.	116

LIST OF FIGURES

NO.	Titles	Page No.
(2.1)	Cementation of soil grains(1) Soil grains (2) Cementing material (3) Porosity (After Harwood, 1988).	9
(2.2)	Regional Distribution of Gypseous Soils in Iraq (After Barazanji, 1973, redrawn by the author).	10
(2.3)	Gypseous Soils Distribution in Iraq at Depths (250-1500 mm) (After Al-Kaabi, 2007).	11
(2.4)	Typical structure of collapsible soil (After Clemence and Finbarr, 1981).	13
(2.5)	Structure of the Collapsible Soils (a) Loaded Structure Before Soaking(b) After Soaking (Houston, et al, 1988).	14
(2.6)	a) SEM for Tikrit Natural Soil. b) SEM After Short-Term Soaking. c).SEM After Subjecting to Single Collapse Test (Schanz and Karim, 2018).	16
(2.7)	Proposed chart for estimation of the (δ) value, (After Aksoy et al., 2016, 2018).	30
(2.8)	Limiting amplitudes for vertical vibration.(after, Richart et al., 1970).	34
(3.1)	Grain Size Distribution Curve for Gypseous Soil: a) S1 (G.C30%), b) S2 (G.C65%).	43
(3.2)	The Results of Compaction Test for Samples Gypseous Soil Used a) for Soil that GC =30% and b) for soil that GC=65%.	47
(3.3)	Results of Single Odometer Collapse Test for Two Samples of Soil Used S1 which GC=30% and S2 which GC=65%.	52
(3.4)	The Principle of the Rotating Mass Type Oscillator	58
(3.5)	The limiting amplitudes for vertical vibration (after, Richart et al, 1970).	65

(3.6)	The testing program.	67
(4.1)	Variation of velocity with time for four value of slenderness ratio, at frequency 10 Hz (dry state).	69
(4.2)	The maximum and minimum velocity versus slenderness ratio at frequency 10 Hz for dry state.	70
(4.3)	Variation acceleration with time for four value of slenderness ratio, at frequency 10 Hz (dry state).	71
(4.4)	The maximum and minimum acceleration versus slenderness ratio at frequency 10 Hz for dry state.	71
(4.5)	Variation of displacement amplitude with time for four value of slenderness ratio, at frequency 10 Hz (dry state).	72
(4.6)	The maximum and minimum displacement amplitude versus slenderness ratio at frequency 10 Hz for dry state.	73
(4.7)	Variation of velocity with time for four value of slenderness ratio , at frequency 10 Hz (soaked state).	74
(4.8)	The maximum and minimum velocity versus slenderness ratio at frequency 10 Hz for soaked state.	75
(4.9)	Variation acceleration with time for four values of slenderness ratio, at frequency 10 Hz (soaked state).	75
(4.10)	The maximum and minimum acceleration versus slenderness ratio at frequency 10 Hz for soaked state.	76
(4.11)	Variation of displacement amplitude with time for four value of slenderness ratio, at frequency 10 Hz (soaked state).	77
(4.12)	The maximum and minimum displacement amplitude versus slenderness ratio at frequency 10 Hz for soaked state.	78
(4.13)	Figure 4.13: a-The Settlement Versus Time for Both States (dry and soaking) at frequency 10 Hz for Different Values of slenderness ratio, b- Zooming of Stage Four.	79
(4.14)	Variation velocity with time for four values of frequency at single solid steel pile (dry state).	81

(4.15)	The maximum and minimum velocity against frequency for dry state at single solid steel pile.	82
(4.16)	Variation of acceleration with time for four values of frequency at single solid steel pile (dry state).	82
(4.17)	The maximum and minimum acceleration versus frequency for dry state at single steel solid pile.	83
(4.18)	Variation of displacement amplitude with time for four values of frequency at single steel solid pile (dry state).	84
(4.19)	The maximum and minimum displacement amplitude versus frequency for dry state at single solid steel pile.	85
(4.20)	Variation velocity with time for four values of frequency at single steel solid pile (soaked state).	86
(4.21)	The maximum and minimum velocity against frequency for soaked state at single steel solid pile.	87
(4.22)	Variation acceleration with time for four values of frequency at single steel solid pile (soaked state).	88
(4.23)	The maximum and minimum acceleration versus frequency for soaked state at single steel solid pile.	88
(4.24)	Variation of displacement amplitude with time for four values of frequency at single steel solid pile (soaked state).	89
(4.25)	The maximum and minimum displacement amplitude against frequency for soak state at single steel solid pile.	90
(4.26)	Figure 4.26: a-The settlement versus time for both states (dry and soaking) for different frequency at single solid steel pile (L/D) 27, b-Zooming of vibration test at soaking State.	91
(4.27)	Variations of velocity with Time to single steel solid pile with Frequency 10 HZ at G.C (30%, 65%) in dry state.	92
(4.28)	Variations of acceleration with Time to single steel solid pile with Frequency 10 HZ at G.C (30%, 65%) in dry state.	93
(4.29)	Variations of displacement amplitude with Time to single solid steel pile with Frequency 10 Hz at G.C (30%, 65%) in dry state.	94

(4.30)	Variations of velocity with Time to single solid steel pile with Frequency 10 Hz at G.C (30%, 65%) in soaked state.	94
(4.31)	Variations of acceleration with Time to single solid steel pile with frequency 10 Hz at G.C (30%, 65%) in soaked state.	95
(4.32)	Variations of displacement amplitude with time to single solid steel pile with Frequency 10 Hz at G.C (30%, 65%) in soaked state.	96
(4.33)	Figure 4.33: a-The settlement versus time for both states (dry and soaking) for two G.C (65,30)% at single steel solid pile, b- Zooming of vibration test at soaking state.	97
(4.34)	Variation of velocity with time for four types of piles (concrete, rough steel ,hollow steel , timber) with frequency 10 Hz (dry state).	98
(4.35)	Variation of acceleration with time for four types of piles (concrete ,rough steel, hollow steel, timber) with frequency 10 Hz (dry state).	99
(4.36)	Variation of displacement amplitude with time for four types of piles (concrete, rough steel, hollow steel ,timber) with frequency 10 Hz (dry state).	100
(4.37)	Variation of velocity with time for four type of piles (concrete, rough steel, hollow steel, timber) with frequency 10 Hz (soaked state).	101
(4.38)	Variation of acceleration with time for four types of piles (concrete ,rough steel, hollow steel, timber) with frequency 10 Hz(soaked state).	102
(4.39)	Variation of displacement amplitude with time for four types of piles (concrete, rough steel, hollow steel, timber) with frequency 10 Hz (soaked state).	103
(4.40)	Figure 4.40: a-The settlement versus time for both states (dry and soaking) for five type of pile at frequency 10 HZ and G.C 30%, b- Zooming of Vibration Test at Soaking State.	104
(4.41)	The relationship between the maximum velocity and slenderness ratio for solid steel pile in dry and soaked state.	105
(4.42)	The relationship between the maximum acceleration and the slenderness ratio (for solid steel pile) in dry and soaked state.	106

(4.43)	The relationship between the maximum displacement amplitude and the slenderness ratio for solid steel pile in dry and soaked state.	107
(4.44)	The relationship between the maximum velocity and the frequency in dry and soaked state.	109
(4.45)	The relationship between the maximum acceleration and the frequency in dry and soak state.	110
(4.46)	The relationship between the maximum displacement amplitude and the frequency in dry and soak state.	111
(4.47)	The relationship between load and settlement with different slenderness ratio a) for L/D 12, b) L/D 17, c) L/D 22 and d) L/D 27.	117

LIST OF PLATES

Plate Title No.	Title	Page No.
(1.1)	Failure of Buildings That are Built on Gypseous Soil (After Abid-Awn, 2010).	3
(2.1)	Driven piles; (a) driven by drop weight, (b) driven by jacking, (after Deeks, et al, 2005).	21
(2.2)	Some types of steel piles (a) I-section, (b) pipe piles.	23
(2.3)	Timber piles (a) driven timber piles, (b) failure of driven timber piles.	25
(2.4)	Concrete piles. Types of concrete piles, (a) cast in situ pile (b) pre-cast pile.	27
(3.1)	Map Image of Tikrit and Study Area with Two Samples Locations (Satellite Imagery from Google Earth).	41
(3.2)	Types of piles used a) solid steel piles , b) hollow steel pile , c) timber pile, d) rough steel pile and e) concrete.	54
(3.3)	View of Steel Model.	53
(3.4)	The Mechanical Oscillator and Cap.	57
(3.5)	Digital Tachometer	59
(3.6)	Checking the Frequency of System Using Digital Tachometer.	59
(3.7)	The piezoelectric accelerometer.	60
(3.8)	Measuring the vibration response.	60
(3.9)	General View of Laboratory Model Testing.	61
(3.10)	Overview of the jack system.	62
(3.11)	The process of compaction soil.	63
(3.12)	Pile insertion process a) putting the plate on the soil surface b) driven the pile in the soil.	64

LIST OF ABBREVIATIONS	
C	Cohesion of soil
C.P.%	Collapse potential of gypseous soil
Cc	Coefficient of curvature
Cu	Coefficient of uniformity
D ₁₀	Grain size at 10% passing
D ₃₀	Grain size at 30% passing
D ₆₀	Grain size at 60% passing
Dr	Relative density of soil
E	Void ratio
E _o	initial void ratio
G.C%	Gypsum content of soil%
G _s	Specific gravity
L/D	Slenderness ratio of pile
N γ , N _q	factors for bearing capacity
\emptyset	Angle of internal friction of soil
S1	Soil one with 30% gypsum content
S2	Soil two with 65% gypsum content
W _c %	Water content of soil
γ_d	Dry unit weight of soil
Δ	Interface friction angle between pile and soil
Δe	void ratio changing in upon wetting
X	Gypsum content
Me	Rotating mass
Ω_0	circular frequency of the system
T.D.S	Total Dissolved Salts
USCS	Unified Soil Classification System
L.L.	Liquid limit
P.L.	Plastic limit
O.M.C.	Optimum moisture content
O.M.	Organic matters

CHAPTER ONE
INTRODUCTION**1.1 General**

Many countries in the world are suffering from gypsum soil. Gypsum is found in a form of $(\text{CaSO}_4 \cdot 2\text{H}_2\text{O})$ or (CaSO_4) . Some countries in Europe and Australia are covered with gypseous soil. The total area of the world is covered by 1.5% of gypsum soil (FAO, 1998). Gypseous soil is found in many area of Iraq from north to south with a percentage range from (10% - 70%) (Ismail, 1994); (AL-Saoudi et al, 2013).

Gypseous soil is very strong when it is dry. Problems can appear when gypseous soils are wet, the dissolution of gypsum content is the main problem of the gypseous soil. Dissolution causes immediate settlement, sudden collapse and reduces the strength of soil under the foundation of structure. Water is found in soil particles either from overflow, top such as rainfall or from the bottom when the level of ground water is rising (Noor et al., 2013). Many solutions are used to decrease the damage of a structure in gypseous soil such as improvement of the soil properties using chemical treatment, physical treatment or using piles.

Deep foundation (pile) is the most common type used in collapsible soils. A full scale is executed of pile load test in collapsible soil, results showed that there is a high reduction in the ultimate bearing capacity of piles due to the immersion of the soil by water (Grigoryan, 1997); (Fernandes and Cintra, 1997).

The experimental studies which are interested in investigating the performance of piles in gypseous soil show that there is a high reduction in bearing capacity and high settlement of piles when flooded after 24 hours, while, in case of dryness, there is a high bearing capacity of piles (Albusoda and Al-Rubaye, 2015; Abd-ullah, 2015).

1.2 Problems with Gypseous Soils

The presence of gypseous soil content under the foundation of a structure is a structural risk, especially when the structure is saturated and this problem makes crack, tilting and collapse of structure (Nashat, 1990). Moreover, the different structures that are built on the gypseous soils are failing in other places such as Tikrit training center, Samarra tourist hotel, Tikrit water tank, Habbanya tourist avillage (Nashat, 1990; Razouki et al, 1994; Al.Muftay, 1997), as shown in plate (1.1).

Furthermore, many cracks are viewed in runway of the Air force college (Al-Neami, 2000). As well, there are a many problems of construction that respect to gypseous soils, such as: collapse, cracks, tilting and leaching (Mahdi, 2004). Additionally, excessive settlement and crack problems are also present in Habbaniya Tourist village, Al Anbar University site and some houses in Al Ramadi city (Tawfeeq, 2009).

Finally, the gypseous soils caused damages in many regions in the world also, in Iraq, such that Arabian countries, USA, Russia, Spain and Chine. The problem accures when water table or rainfall fractures and infiltrates into gypseous soils (Al-Soudi et al. 2013).



Plate (1.1) Failure of Buildings That are Built on Gypseous Soil (After Abid-Awn, 2010).

1-3 Dynamic Loading

The behavior of soils related to dynamic load has caused the problem, so problems appear when a genuine simulation of site condition is necessary to be investigated. Seismic activity, machine foundation, explosion, traffic and rail classified as a source of dynamic loading and these caused vibrations through the soil. Most of the soil properties are effected when exposed to vibration (Barkan, 1960).

It is important to investigate the effect of vibrated loading in soil properties, because most of the geotechnical engineer for designing a project of civil engineering such a building foundation, evaluating the stability of slopes and dams. In different cases, the soil that is stable under static load fails, when insecure to dynamically load.

There is numerous number of structure have a variable, such towers, pumping stations turbines on or underground surface, therefore foundation and under beneath soil are insecure to wide range of dynamic loading during different frequencies which are excited by nature of the structure, state loading may possibly diverge from large number of cycle with a low strain amplitude in state of vibration due to device to compare the small number of cyclic of huge strain amplitude from seismic activities (Silver and Seed, 1971).

Piles (deep foundation) used for hundreds of years, but in the last years or have seen a noticeable increase in an interest in pile dynamic. So piles are used widely in an important area of application have protrude, such, nuclear power plants and offshore towers, piles have frequently failed in earthquakes or were damaged, liquefaction, vibration effects and embankment movement caused damage in piles. Also piles damage during earthquakes in Japan given by mizuno 1987, it appeared also in Alaska earthquake of 1964 and Loma priera earthquake of 1989.

1-4 Types of Dynamic Loads

The resources of dynamic load that mentioned by Rao, (2011).

- 1- Earthquakes.
- 2- impact loads.
- 3- Forces generated by wind .
- 4- Vicinity to vibration environment .

5- Moving load.

6- Machines, which content unbalanced rotating and reciprocating parts and dynamic load and produce transient.

7- Periodic force and moments as an example due to mining and piling operation, sonic boom and drilling.

1.5 Types of Machines

Prakash and Puri (2006) classified the machines according to type of periodic forces created by these machines. The very important type are:

1- Impact machines: these impact loads created by machines such as forging hammers and the speed usually from 60 – 150 blows per minute in short interval and practically die out.

2- Reciprocating machines: these machines generate periodic unbalanced forces (such as steam engines). In these machines, the operation speeds usually less than (600 r.p.m).

3- Rotary machines or high speed machines such as the rotary compressors or turbo – generators and the speed, operation is more than (3000 r.p.m) and up to (12000 r.p.m).

1.6 Objective of the Study

The aim of this study is to explain the behavior of single pile subjected to dynamic load in gypseous soil. In this study different (L/D) from piles and different types are used of it and the all experiments tested in both cases (dry and soaked) condition.

1.7 Layout of the Study

Chapter one: includes the introduction and describing the soil that is used in experimental and piles also the dynamic load that affect piles and types of machine, object of study.

Chapter two: contains the literature reviews of the post student and researches that are in relation of dynamic response of pile under the effect of vertical vibration in dry and soaked state.

Chapter three: this chapter includes the experiment work such as material properties and the classification of soil that used and description of the model and testing program.

Chapter four: this chapter includes the results of tests and their discussions.

Chapter five: includes the main points concluded from this work and recommendation for future work.